International Journal of Mining and Geo-Engineering

Developing new Adaptive Neuro-Fuzzy Inference Systems to predict granular soil groutability

Mostafa Asadizadeh^{a,*}, Abbas Majdi^b

^a Department of Mining Engineering, Hamedan University of Technology, Mardom Street, Hamedan, Iran ^b School of Mining Engineering, University of Tehran, Tehran, Iran

Article History:

Received: 02 April 2018, Revised: 12 September 2018 Accepted: 27 September 2018.

ABSTRACT

Three Neuro-Fuzzy Inference Systems (ANFIS) including Grid Partitioning (GP), Subtractive Clustering (SCM), and Fuzzy C-means clustering Methods (FCM) have been used to predict the groutability of granular soil samples with cement-based grouts. Laboratory data from related available in the literature was used for the tests. Several parameters were taken into account in the proposed models: water:cement ratio of the grout, the relative density of the soil, grouting pressure, soil and grout particle size dimensions named D_{15xoil} , D_{10xoil} , $d_{85\,grout}$ and $d_{95\,grout}$ and the percentage of the soil particles passing through a 0.6 mm sieve. The accuracy of ANFIS models was examined by comparing these models with the results of experimental grout-ability tests. A sensitivity analysis showed that the ratios of D_{15xoil} / $d_{85\,grout}$ and D_{10xoil} / $d_{95\,grout}$ were the most effective parameters on the groutability of granular soil samples with cement-based grouts and the grout water:cement ratio of the grout was determined as the least effective parameters.

D

Keywords : Groutability, ANFIS, Clustering Algorithm, Granular soil

1. Introduction

Grouting is a process whereby some external materials are injected into soil pores and rock fissures to improve the mechanical and hydraulic properties of the media for short- and long-term engineering needs. Generally, the process involves injecting a viscous grout mixture under the pressure into the porous media or joints until they are blocked by the larger grout particles. Permeation grouting has been commonly used in ground improvement techniques, it has also been used extensively to increase the liquefaction resistance of existing structures because it has a low grouting pressure (P) [1].

Groutability (N) of grouts is defined in terms of its capability for injection into a target soil or rock to improve the mechanical properties of materials or to reduce their permeability [2-6]. An important issue for application of cement-based grout is the trustworthy prediction of groutability in different media. The grouting process relies on complicated time-dependent transportation of cement particles right through the other side of the soil or rock materials. This prediction process requires several variables such as the distribution of grain size in the grout and soil materials, pore size and hydraulic conductivity of the soil, the injection pressure, and the viscosity of the grout suspension. This complicates the predictions meaning that no universally comprehensive set of criteria or methodology have been determined. Many researches have been carried out on tests for soil groutability predictions. Some of these have presented basic empirical equations related to the size of the soil and cement particles [7,8,9-20]. In practice, empirical criteria are regularly applied as the main tools to determine the groutability ratio of soil samples. A primary research on groutability predictions of granular soil samples includes an analogy between the particle dimension of the host soil and cement grout. It was reported that the penetration zone by the grout mixture in a soil sample is limited by the grain size distribution of the soil [8]. However, the research on grout penetrability has reported that an important consideration is the size of the voids being grouted in relation to the size of the solid particles in the grout [12]. Groutability was defined as the ratio (GR) for grouting natural soil formations as Equation 1.

$$GR = D_{15 \text{ soil}} / D_{85 \text{ grout}} > 25$$
 (1)

Where D_{15} corresponds to the sieve diameter through which 15 wt % of the soil sample passes or 15 wt% of the soil sample is finer than the grain size diameter, and D_{85} represents the grain size diameter below which 85 wt % of the grout mixture is finer. According to [16], the first requirement for the selection of grout is that its particle size should be smaller than the dimensions of the voids to be filled, which is determined by Equation 2.

$$_{15 \text{ soil}}/D_{85 \text{ grout}} > N$$
 (2)

Where N ranges from 5 to 20 proposed for clay grouts and depends on local conditions. It was further added that the second and third (the last) requirements are stability and pumpability of a grout sample, respectively. However, in [13], the major controllable variables affecting the efficiency of injection are as follow: pumping rate, setting time of the grout, pumping time at a given pipe location, the distance-time schedule for pulling or driving the pipe, grout viscosity, grouting pattern, and possible inter-relationships among the parameters. Additionally, in [15], the major uncontrollable variables were groundwater flow and stratification. According to [14], grouting with cement is not possible when the sand:grout ratio, $D_{15sand} / D_{15grout}$ is below 11. Grouting is

only possible when the sand:grout ratio, D_{15sand} / $D_{15grout}$ is more

^{*} Corresponding author. E-mail address: m.asaadizadeh@hut.ac.ir (M. Asadizadeh).



than 24. For an effective injection, the soil pore space should be three times of the diameter of the grout particle to avoid the blockage by bridging (Kennedy, 1962; King & Bush, 1963). Based on experimental data, the U.S. Army Corps of Engineers Waterways Experiment Station proposed that grouting could be successfully accomplished if the grout ability ratio was greater than N = 15 [15], as shown in Equation 3.

$$N = D_{15 \text{ soil}} / D_{85 \text{ grout}} \ge 15$$
 (3)

According to [21], the groutability ratio is defined as the ability of grout particulates (soil, cement, clay) to penetrate a soil formation. For successful grouting of a soil:

$$GR = D_{15 \text{ soil}} / D_{85 \text{ grout}} \ge 25 \tag{4}$$

The soil grain size limits the penetrability of the grout mixture [22]. So that, in a soil sample, if more than 10 wt% of particles passes through sieve number 200, the soil sample is not considered groutable. According to [9], the dimensions of the voids exist within the soil particles compared to those of the grout mixture particles and is considered as a logical criterion. Furthermore, some 'groutability ratios' that have been proven useful for soil samples were presented as follows:

$$\begin{vmatrix} \mathbf{N} = \mathbf{D}_{15 \text{ soil}} / \mathbf{D}_{85 \text{ grout}} \\ \mathbf{N}_c = \mathbf{D}_{10 \text{ soil}} / \mathbf{D}_{95 \text{ grout}} \end{aligned}$$
(5)

When N>24, grouting is permanently feasible and when N<11, grouting is not practicable. As well, when $N_{\rm c}>11$, grouting is permanently feasible and when $N_{\rm c}<6$, grouting is not practicable [23]. A method was developed and used to measure the complicated structure of pore spaces [24]. According to [25], permeation is controlled by the size of particle than by viscosity and cohesion. It was added that the size of the pores of granular soil and aperture of rock fissures were dominant controls over groutability. While the geometry of a fissure is relatively

simple to model, the pore system of loose soils is complex. Based on the results of cement injected sands and gravel samples [11], groutability ratios are suggested to be as shown in Equation 6. $[N = D_{res} = /(D_{res} = >10]$

$$N = D_{10 \text{ soil}} / D_{90 \text{ grout}} > 10$$

$$N = Dm_{\text{soil}} / D_{90 \text{ grout}} > 3$$

$$(6)$$

In which, Dm is the size of soil voids. An empirical the groutability ratio of granular soil was proposed for consolidation grouting as a function of grain size, relative density and fine contents of soil, the dimension of cement particulates, water:cement ratio of the grout mixture and the grouting pressure [7]. The performance of conventional groutability criteria which are based on the groutability ratios can be considered as optimistic. According to [17], test groutability formulas do not consider parameters such as the characteristic grain sizes of grout and soil, water-cement ratio, type and percentage of used additives, and sand density. Well-designed formulations through the proper application of additives reduce the cost of a grouting operation via decreasing the cohesion and increasing the penetrability of the grout [19]. Decreasing the cohesion of a grout mixture, whenever keeping a consistent grout mixture, which boosts its penetration, has been reported in other researches [26]. However, it is reported in [27] that groutability was also enhanced by using flash, while permeability showed a significant decrease. Moreover, the possible cement grout penetration, investigation of the influence of grout viscosity variation and filtration were studied in [28]. Artificial inference systems have been recently developed to solve complicated problems. Systems such as neural networks and fuzzy logic have been used to solve many geotechnical difficulties in recent years. Such systems and their associated methods have advantages and some disadvantages. The advantage of artificial neural networks is pattern recognition and the capacity of adapting a method to cope with changing environments. Fuzzy logic has the advantage of incorporating human knowledge and expertise to deal with uncertainty and imprecision. Therefore, many efforts have been made to take the advantage of both of these approaches. As a result of these studies, many investigators have recently suggested the application of a combination of these approaches termed as the ANFIS method [29-40]. In the case of permeation grouting, some

researchers utilized an Artificial Neural Network (ANN) for predictions of soil groutability [41]. The researchers reported that classical groutability prediction formulas, which are mainly contingent upon the grain-size of the soil and the grout, were not suitable for seminanometer scale grout. They found that the accuracy of the proposed formulas varied from 45% to 68%, a domain that is not suitable for practical purposes. Unlike this, an ANN model was proposed by [42] to predict the groutability of granular soils by cement-based grouts, utilizing grouting pressure, water:cement ratio of the grout, the diameter of sieve through which 15% of soil particles and 85% of the grout pass, and the relative density of the soil. It was reported that high success rates exceeding 90% for some existing empirical methods and a highly successful prediction ratio (95.4%) was obtained using the ANN models. According to [43], the water:cement ratio of the grout was the most effective parameter on the dynamic response of grouted sands and the influence of cement grain size and cement pozzolan ingredient were secondary, but not insignificant.

In this paper, three known ANFIS models are developed to evaluate the groutability of granular soil samples with cement grout; these include GP, SCM, and FCM. To fulfill this goal, datasets of 87 laboratory-grouting tests were employed using the data available in the related literature. The efficiency of ANFIS models was compared with the test results. Since it was not clear which of the above-mentioned ANFIS methods had the better performance in terms of addressing the grouting problems, a comparison was carried out to compare the performance of the three models to determine the best performing model. Furthermore, a sensitivity analysis was performed to distinguish the most effectual input variables on the groutability of granular soil samples.

2. Background study

2.1. Takagi-Sugeno fuzzy system

The theory of fuzzy sets was presented by Zadeh to cope with problems that have uncertainty due to ambiguity and imprecision [44]. The fuzzy set theory accurately investigates ambiguous conceptual phenomena using a precise mathematical framework. This theory is an appropriate modeling language for imprecision and ambiguous theoretical criteria, phenomena, and relations [45]. The process of formulating the mapping from an input to an output utilizing fuzzy logic is called fuzzy inference. The fuzzy set theory can decode the enigmatic states of reasoning utilized in an environment defined by uncertainty and ambiguity. Fuzzy logic is the system of concepts, rules, and approaches applied to rough reasoning using the fuzzy set theory [46]. Fuzzy logic uses a list of 'if-then' statements called rules to map an input domain to an output domain. The general form of rules is as follows:

if w is C then z is D

in which *w* and *z* are variables in domains W and Z; C and D are fuzzy sets based on W and Z, respectively. In this rule, the antecedent 'is if' part of the sentence and the consequence is the 'then' part of the sentence [46]. Applying the information verbalized in the form of natural linguistic statements is the prominent feature of rule-based fuzzy logic. Membership function (MF) is the main concept of fuzzy logic, and numerically, it states the degree to which a given element belongs to a fuzzy set. Some methods such as expert judgment or data analysis can be used to apply the number of MF, location, and shape to the fuzzy model [46]. A subcategory of model designation that copes with creating a fuzzy logic is fuzzy modeling. The response of an unfamiliar system specified using some sample data can be foreseen and explained using a fuzzy inference system [47]. A fuzzy interference system is a world-renowned computing system, which is contingent upon notions of fuzzy logic. Different parts of a Fuzzy Inference System (FIS) are presented in Table 1. Furthermore, a schematic diagram of FIS is presented in Fig. 1 [48].

Different applications of FIS have been published so far. The most prevalent fuzzy models used are the Mamdani, Takagi–Sugeno–Kang (TSK), Tsukamoto Singleton [49]; and the TS fuzzy model introduced by Takagi and Sugeno [50].

Table 1. The different parts of a fuzzy inference system [48].

Different Parts of a fuzzy	The responsibility of different Parts of a fuzzy inference system
Fuzzification unit	Converting crips input variables into a fuzzy amount
Rule base	Including fuzzy if – then rules
Database	Description of fuzzy sets membership functions
Decision-making unit	Performing inference operations on the rules
Defuzzification unit	Transforming fuzzy results into an output

The logic structure of the TSK model is presented as follows [51]: if x is $A_1 \& y$ is B_1 then $f_1 = p_1 x + q_1 y + r_1$ (7)



Fig. 1. Schematic structure of a FIS model with a crisp output [52].

2.2. Basic Concept of ANFIS

FIS is able to model the inference procedure and linguistic features of human understanding without applying accurate quantitative investigations. ANNs are a combination of many interdependent processing components that are comparable to neurons. A collection of data is imported to the training algorithm of ANN and the output is checked for the desired result through this algorithm. In this method, the human process of decision-making is intelligently imitated by a combination of ANN and FIS. In conventional ANN, just weight quantity alters throughout the learning phase, whereas in a neuro-fuzzy decision-making system, the learning capability of ANN is coupled with the reasoning process of FIS [53]. ANFIS has been introduced as an Adaptive Neuro-Fuzzy Inference System. Basically, ANFIS utilizes a FIS and adjusts it using a backpropagation algorithm and employing a set of input-output data. The combination of FIS and ANN enables FIS to learn. The structure of an adaptive neural network includes several nodes joined via oriented links. A node function with unchangeable or adaptable parameters defines each node. Neural network algorithms, when FIS is loaded, can be applied to calculate unknown factors and this decreases the error values, as traditionally described for every parameter of the model and this optimization process makes the model adaptive [54].

ANFIS usually utilizes amalgamation of backpropagation for the purpose of learning the presupposition parameters and the least mean square for determination of resulting parameters. A stage of the learning process has two phases: the first phase involves the propagation of input patterns and applies the iterative least mean square process to evaluate the optimal ending parameters, whilst ancestor parameters are supposed not to change for the present phase throughout the training set. Furthermore, the second phase is included in the repeated propagation of patterns and in this epoch, backpropagation is utilized to adjust the ancestor parameters, whilst the ending parameters are maintained. This process is followed by iteration (Fig. 2a). For instance, we considered a FIS that has two inputs **x** and **y** and one output **z**. Therefore, two fuzzy 'if-then' rules of Takagi and Sugeno's type are presented in Equation 8 [52].

Rule 1: If x is
$$A_1$$
 and y is B_1 , then $f_1 = p_1 x + q_1 y + r_1$
Rule 2: If x is A_2 and y is B_2 , then $f_2 = p_2 x + q_2 y + r_2$ (8)

The adaptive neural network and its operationally identical to FIS are presented in Figs. 2a and 2b respectively.

In this study, a neuro-fuzzy model was utilized containing five layers [55], as follow:

In the first layer; every node i creates a membership grade of a lingual

ticket. For example, the node function of the ith node could be as Equation 9, [52]:

$$Q_i^{l} = \boldsymbol{\mu}_{Ai}(\mathbf{x}) = \exp\left\{-\left(\frac{\mathbf{x} - c_i}{a_i}\right)^2\right\}$$
(9)

in which, x is the input to node i, and A_i is the lingual ticket (small, large, etc.) introduces this node and $\{a_i, b_i, c_i\}$ is the parameter collection that alters the MF configuration. The parameters of the first layer are named "*premise parameters*".



Fig. 2. (a) Schematic structure of the TSK fuzzy model; (b) ANFIS model Structure [52].

In the second layer, every node in computes the "*firing strength*" of each rule by multiplication described in Equation 10, [52]:

$$\mathbf{Q}_{i}^{2} = \mathbf{w}_{i} = \boldsymbol{\mu}_{Ai}(\mathbf{x}) \times \boldsymbol{\mu}_{Bi}(\mathbf{y}), \ i = 1, 2.$$

$$(10)$$

In the third Layer; the i th node computes the ratio of the *i*th rule's firing strength to the aggregate firing strengths of all rules as defined in Equation 11, [52].

$$Q_i^3 = \bar{w}_i = \frac{W_i}{W_1 + W_2}, i = 1, 2.$$
 (11)

The outputs of the third layer are named "*normalized firing* strengths".

In the fourth layer, the node function of each node *i* is presented in Equation 12, [52].

$$Q_i^4 = \overline{w}_i f_i = \overline{w}_i (p_i x + q_i y + r_i)$$
(12)

in which $\overline{w_i}$ is the third layer output. Parameters in the fourth layer are called '*consequent parameters*'.

In the fifth layer, an individual node shown by a circle calculates the *'overall output'* as the aggregate of all interring signals. The node function is represented by Equation 13.

$$Q_i^5 = \text{Overal Output} = \Sigma \overline{w}_i f_i = \frac{\Sigma_i w_i f_i}{\Sigma_i w_i}$$
(13)

3. Data set

As discussed in section 2, all techniques in the literature that predict groutability depend on an analogy between the distribution of grain size and permeability of host soil and cement grout. Nonetheless, some researchers have shown that many other parameters also affect groutability [27,43,56]. In addition, it has been agreed that a better estimation of groutability is made when many effective soil and grout variables are included. Hence, the excellent ability of ANFIS in



parameter identification can be applied to boost the forestall-ability of the model. Therefore, a set of information including experimental tests is employed to develop the well-made ANFIS models. The tests consist mainly of water to cement proportion of the grout mixture (W/C), the amount of soil which is less than 0.6 mm (FC), the relative density of soil (Dr), the pressure under which grout is injected (P) and the grainsize distribution of the host soil and the grout. Based on [57], an investigation was done to determine the groutability of sand samples with various grain-size distribution curves using micro fine cement samples. In that study, fifteen experiments were accomplished by injecting the grout mixture under 100 kPa pressure into the sand with a given relative density. Furthermore, in [42] the same test setup was used to carry out sixteen grouting tests. Based on [7], 38 grouting tests were performed on granular soil samples. However, eighteen experimental grouting tests were accomplished on granular soil samples [58]. All datasets were collected from the literature, and the ANFIS models were established according to a total of 87 experiments involving W/C, Dr, P, FC, NI = $D_{15 \text{ soil}}/d_{85 \text{ grout}}$, N2 = $D_{10 \text{ soil}}/d_{95 \text{ grout}}$ and the results of the grouting experiments. All data sets are presented in Table 2 and in order to investigate the exact details of the experimental procedure, related references are presented as follows [58]:

			I adle 2.	Data sets	of groutability of	collected from [7,4	2,57,58].	
No	W/C	Dr(%)	P(kPa)	FC(%)	$N1 = \frac{D_{15 \text{ soil}}}{1}$	$N2 = \frac{D_{10 \text{ soil}}}{1}$	Experimental	Reference
					u _{85 grout}	u _{95 grout}	Groutability	
1	1	80	50	1	87.10	41.67	1	[19]
2	1	80 80	100	1	45.16	21.33	1	[19]
4	1	80	100	100	11 29	5 33	0	[17]
5	1	80	150	100	11.29	5.33	ů 0	[19]
6	1	80	250	100	11.29	5.33	0	[19]
7	2	30	100	100	11.29	5.33	0	[19]
8	2	30	200	100	11.29	5.33	0	[19]
9	1	80	100	34	14.19	6.17	0	[19]
10	1	80	150	34	14.19	6.17	0	[19]
11	1	80	200	34	14.19	6.17	0	[19]
12	2	30	100	34	14.19	6.17	0	[19]
13	2	30	200	34	14.19	6.17	0	[19]
14	1	80	100	33	27.42	11.67	1	[19]
15	1	80	100	15	19.35	8.00	0	[19]
10	1	80	200	15	19.35	8.00	0	[17]
18	1	30	200	15	19.35	8.00	0	[19]
19	2	30	200	15	19.35	8.00	0	[19]
20	1	80	100	15	19.35	9.00	0	[19]
21	2	30	200	15	19.35	9.00	0	[19]
22	1	80	100	10	23.55	10.00	0	[19]
23	1	80	100	5	28.39	12.67	1	[19]
24	1	80	100	5	28.39	12.67	0	[19]
25	1	80	150	5	28.39	12.67	0	[19]
26	1	80	200	5	28.39	12.67	0	[19]
27	1	30 20	200	5	28.39	12.67	0	[19]
20 29	2	30	100	5	20.37	12.67	1	[19]
30	3	30	100	5	28.39	12.67	0	[19]
31	2	30	200	100	11.29	5.33	ů 0	[19]
32	2	30	200	34	14.19	6.17	0	[19]
33	1	80	200	15	19.35	8.00	0	[19]
34	2	30	200	15	19.35	8.00	0	[19]
35	1	80	100	5	28.39	12.67	1	[19]
36	1	80	150	5	28.39	12.67	0	[19]
37	1	80	200	5	28.39	12.67	1	[19]
38	1	30	200	5	28.39	12.67	1	[19]
39 40	4	70	517	100	28.33	18.75	1	[58]
41	4	70	517	100	25.00	16.25	1	[58]
42	4	70	690	100	25.00	16.25	0	[58]
43	4	70	690	100	21.67	15.00	0	[58]
44	4	70	483	25	71.67	43.75	1	[58]
45	4	70	517	25	66.67	40.00	1	[58]
46	4	70	690	25	25.00	15.00	1	[58]
47	4	70	690	25	58.33	37.50	0	[58]
48	2	70	655	100	28.33	18.75	1	[58]
49	4	70	245	100	28.33	18.75	1	[58]
50	2	70	552	65	26.55	16.75	1	[50]
52	4	70	483	65	36.67	23.75	1	[58]
53	6	70	241	65	36.67	23.75	1	[58]
54	2	70	552	25	71.67	43.75	1	[58]
55	4	70	448	25	71.67	43.75	1	[58]
56	6	70	241	25	71.67	43.75	1	[58]
57	1	30	100	100	13.64	6.67	0	[57]
58	1	27	100	20	121.59	69.33	1	[57]
59	1	30	100	19	57.95	24.00	1	[57]
6U 61	1	52 30	100	23 29	40.40 34 ng	10.67	1	[57]
62	1	30	90	19	57.95	24.00	1	[57]

Table 2. Data sets of groutability collected from [7,42,57,58].

63	1	30	100	23	45.45	18.67	1	[57]
64	1	30	100	29	34.09	15.33	1	[57]
65	1	30	100	33	30.68	13.33	0	[57]
66	1	30	100	38	25.00	11.33	0	[57]
67	1	30	100	19	57.95	24.00	1	[57]
68	1	30	100	23	45.45	18.67	1	[57]
69	1	28	100	29	34.09	15.33	1	[57]
70	1	31	100	33	30.68	13.33	1	[57]
71	1	30	100	38	25.00	11.33	0	[57]
72	1	30	490	100	146.67	83.33	1	[42]
73	1	30	490	77	160.00	83.33	1	[42]
74	1	30	490	69	173.33	91.67	1	[42]
75	1	30	490	62	186.67	100.00	1	[42]
76	1	30	490	54	200.00	108.33	1	[42]
77	1	30	490	46	266.67	116.67	1	[42]
78	1	30	490	23	760.00	433.33	1	[42]
79	1	30	490	62	186.67	100.00	1	[42]
80	1	30	490	69	173.33	91.67	1	[42]
81	1	30	490	77	160.00	83.33	1	[42]
82	1	50	490	54	200.00	108.33	1	[42]
83	1	50	490	62	186.67	100.00	1	[42]
84	1	50	490	69	173.33	91.67	1	[42]
85	1	60	490	46	266.67	116.67	1	[42]
86	1	60	490	54	200.00	108.33	1	[42]
87	1	60	490	62	186.67	100.00	1	[42]

4. Application of ANFIS to predict groutability

As discussed in section 4, the Adaptive Neuro-Fuzzy Inference System is a FIS that has to be initialized one at a time. There are techniques for structure recognition to establish a prime ANFIS structure prior to the application of any parameter-adjusting mechanism.

Structure recognition in fuzzy modeling includes the following parts [52]:

- Selecting pertinent input parameters;
- Dividing of input domain;
- Quantifying MFs for every input variable;
- Quantifying if-then rules;
- Ancestor statement of fuzzy rules;
- Result statement of fuzzy rules.
- Selecting primary factors of MFs

In this paper, in order to recognize the ancestor MFs, the following three ANFIS models have been utilized:

- 1- Grid partitioning (GP);
- 2- Subtractive Clustering Method (SCM);
- 3- Fuzzy C-means clustering Method (FCM).

4.1. Grid Partitioning (GP)

Autonomous divisions of every ancestor parameter are proposed in the Grid Partitioning (GP) method [52]. For the purpose of developing a model, the MFs of all ancestor parameters can be defined by an expert and by applying former experience and understanding. The essence of linguistic phrases in an apparent text is represented by means of designed MFs. However, in plenty of organizations, particular understanding is not accessible in these divisions. In this method, the spaces of ancestor parameters are easily divided into a number of MFs with equal space and shape. The MFs parameters can be optimized by means of the available input-output data.

4.2. Subtractive Clustering Method (SCM)

SCM was originally suggested by [54]. In this method, data are considered as nominations for the central point of a cluster. The SCM algorithm is presented below:

First, imagine a set of *n* data points $\{x_1, x_2, x_3, ..., x_n\}$ in an Mdimensional domain. SCM calculates this matter utilizing data points as nominations for cluster centers. In view of the fact that every data point is a nomination for a cluster center, the function of density measure at a given data point x_1 is presented as Equation 14.

$$D_{i} = \sum_{j=1}^{n} exp\left(\frac{\left(-\left\|x_{i} - x_{j}\right\|^{2}\right)}{\left(\frac{r_{a}}{2}\right)}\right)$$

(14)

in which r_a is a positive constant. If a data point has lots of neighboring data, it will have a high-density value. The neighborhood is defined by the radius r_a ; all data points beyond this radius grant just moderately to the density measure. When the density function of every data point is calculated, the maximum density will be chosen as the first cluster center. Following the calculation of the density function of every data point, with the maximum density measure is chosen as the first cluster center. If x_{el} -the first cluster center- is selected as the data containing a higher density amount D_{el} , the density measurement of every data point x_i will be updated as Equation 15.

$$\mathbf{D}_{i} = \mathbf{D}_{i} - \mathbf{D}_{ci} \exp\left(-\left(\frac{\left(-\left\|\mathbf{x}_{i} - \mathbf{x}_{cj}\right\|^{2}\right)}{\left(\frac{\mathbf{r}_{b}}{2}\right)^{2}}\right)^{2}\right)$$
(15)

in which r_b is an affirmative fixed amount. When the density computation was updated for every data point, the subsequent cluster center x_{c2} will be chosen. Finally, the whole density computations for data points are updated reiteratively. This calculation is iterated up to create an adequate quantity of cluster centers.

4.3. Fuzzy C-means clustering Method (FCM)

It should be noted that FCM is based on Hard C-means clustering (HCM). The main difference between FCM and HCM is that in FCM all data is placed in a cluster with a degree of membership, provided that in HCM all data is rightly placed in a specific cluster or not. This algorithm was proposed by [18]. FCM divides a set of *n* vector x_i , i = 1, 2, ..., n into fuzzy sets, and identifies the center of each cluster by minimizing the cost function of incongruity measure. i = 1, 2, ..., c are arbitrarily chosen from the *n* points. The FCM algorithm is explained as follows: at first, the cluster center c_i , i = 1, 2, ..., c are selected arbitrarily out of *n* data points $\{x_1, x_2, x_3, ..., x_n\}$. The elements of membership

с



matrix U can vary from 0 to 1. However, as presented in Equation 16, the aggregate of membership degree of a data point to all clusters is unity.

$$\sum_{i=1}^{c} u_{ij} = 1 \quad \forall j = 1, ..., n$$
 (16)

Then, the membership matrix U is computed utilizing Equation 17.

$$u_{ij} = \frac{1}{\sum_{k=l}^{c} \left(\frac{d_{ij}}{d_{kj}}\right)^{2/m-l}}$$
(17)

In which $d_{ij} = \|c_i - x_j\|$ is the Euclidean interval of the *t*th cluster center from the *t*th data point, $d_{kj} = \|c_k - x_j\|$ is the Euclidean interval of the *k*th cluster center from the *t*th data point, and *m* is the fuzziness index.

Subsequently, the cost function is computed based on Equation 18. If the cost function is less than a specific limit, the process will stop.

$$J(U, c_1, ..., c_c) = \sum_{i=1}^{c} J_i = \sum_{i=1}^{c} \sum_{j=1}^{n} u_{ij}^m d_{ij}^2$$
(18)

Finally, new fuzzy cluster centers c_i , i = 1, 2, ..., c are computed utilizing Equation 19.

$$_{i} = \frac{\sum_{j=1}^{n} u_{ij}^{m} x_{j}}{\sum_{j=1}^{n} u_{ij}^{m}}$$
(19)

5. Groutability prediction utilizing ANFIS model

In this study, three types of modeling producer were carried out to foretell groutability for granular soil samples. ANFIS models including GP, SCM, and FCM were applied to predict the groutability of granular soil samples with cement-based grouts. According to the GP model, the fuzzy rule-base for the Sugeno model was specified in MATLAB software utilizing the ANN model presented in [52]. The schematic design structure of ANFIS is illustrated in Fig. 3. Calculations for this study were conducted using Matlab R2010a software and all runs were carried out on a Laptop with Intel Core 2 Duo @2 GHz CPU and 2 GB RAM. Depending on the chosen method for the ANFIS models, the run time of each model varied from 2 to 5 minutes.



Fig. 3. Shows a schematic of design structures for the GP, SCM and FCM ANFIS models.

As illustrated in Fig. 3, the parameters of W/C, Dr, P, FC, NI, and N2 were introduced to the ANFIS models as input parameters and just the 'a' output parameter, i.e. groutability, was acquired. Specifications of groutability data are presented in Table 3.

Table 3. Specifications of groutability data applied to ANFIS models.

Parameter	Description	Symbols	Range
Input	Water/cement ratio of the	W/C	1-6
	grout		
	Relative density of the host	Dr	27-80
	soil (%)		
	Grouting pressure (kPa)	Р	50-690
	Content of soil passing	FC	1-100
	through a 0.6 mm sieve (%)		

	$\mathrm{D}_{15~base~soil}/~\mathrm{d}_{85~cement~grout}$	N1	10-762
	$D_{10 \text{ base soil}} / d_{95 \text{ cement grout}}$	N2	4-433.33
Output	Groutability	Groutability	0 or 1

The training phase for each model applied 62 data collection, randomly selected from a database. For the purposes of having a more effective training phase, the normalization of datasets was carried out to the domain of [0,1] by Equation 20.

$$x_{\text{normalized}} = \frac{x - x_{\min}}{x_{\max} - x_{\min}}$$
(20)

in which x is an input variable and x_{min} and x_{max} are minimum and maximum values of each variable, respectively.

In the grid partitioning ANFIS model, for every input parameter, two Gaussian fuzzy collections were adapted and 64 fuzzy rules were created in its network structure. Detailed properties of the GP model are presented in Table 4.

Table	4.	Types	of	paramete	ers ar	ıd th	e amo	ounts	appl	ied	to	train	the	AN	FIS
						mc	dels.								

ANFIS parameter type	ANFIS	ANFIS	ANFIS
	(GP)	(SCM)	(FCM)
MF type	Gaussian	Gaussian	Gaussian
Output MF	Linear	Linear	Linear
Number of nodes	161	345	177
Number of linear parameters	448	168	84
Number of nonlinear parameters	24	288	144
Total number of parameters	472	456	228
Number of training data pairs	62	62	62
Number of testing data set	25	25	25
Number of fuzzy rules	64	24	12

The ultimate (trained) Gaussian shaped MFs of the input variables are presented in Fig. 4.



Fig. 4. Ultimate MFs of the grid partitioning method.

The ANFIS model based on SCM, includes 24 rules and its specification is presented in Table 4. The ultimate (trained) MFs of input variables are illustrated in Fig. 5.

1



Fig. 5. Ultimate MFs of subtractive clustering method.

The last ANFIS model, which is based on FCM contains 12 rules. The MFs of the input variables are illustrated in Fig. 6. Details of the parameters of the ANFIS (FCM) model are presented in Table 4.



Fig. 6. Ultimate MFs of Fuzzy C-means clustering method.

6. Prediction performance

In order to calculate the prediction capability of the proposed models, an efficiency evaluation of the ANFIS models has been made using the test data. The test data are almost 25 data that were not utilized in the learning process of the model. It should be noticed that the test data were chosen randomly. Ten samples of testing data are presented in Table 5.

6.1. Performance assessment of ANFIS models

The precision of the ANFIS models is tested through comparing and contrasting the model predictions with the results of groutability tests acquired through grouting experiments. Since the results of modeling and the measured data were discreet i.e. 0 (ungroutable) or 1 (grout

able), in order to report the performance of the models, success, and failure rates were defined as Equations 21 and 22 respectively. **Table 5.** Sample of data used in testing phase [7,42,57,58].

No.	W/C	Dr (%)	P (kPa)	FC (%)	N1	N2	Groutability	
1	1	80	100	100	10.00	4.00	0	
2	2	30	100	34	14.19	6.17	0	
3	2	30	100	5	28.39	12.67	1	
4	4	70	517	100	26.67	17.50	1	
5	1	80	250	100	10.00	4.00	0	
6	4	70	690	100	21.67	15.00	0	
7	2	30	200	34	14.19	6.17	0	
8	2	30	100	5	28.39	12.67	1	
9	1	80	100	33	23.00	9.00	1	
10	4	70	690	25	25.00	15.00	1	
Success rate (%) = $\frac{Correct \ prediction}{Case \ records} \times 100$								

Failure rate (%) =
$$\frac{Incorect \ prediction}{Case \ records} \times 100$$
 (22)

As shown in Table 6, the average correct prediction cases of the ANFIS models, GP, SCM, and FCM, are 24.33, 25 and 24 out of 25 testing cases, respectively. For the purpose of cross-validation, the set of 87 data were separated into two groups, one of them 62 and the other one 25, as the training and testing datasets, respectively. The process was applied to three different arrangements. The test data were completely different in each arrangement. The results of applying these three datasets to the models are presented in Table 6.

Data coto	ANFIS	Case	Incorrect	Correct	Failure	Success
Data sets	Model	Records	prediction	prediction	rate (%)	rate (%)
	GP	25	1	24	4	96
1	SCM	25	0	25	0	100
	FCM	25	3	22	12	88
	GP	25	1	24	4	96
2	SCM	25	0	25	0	100
	FCM	25	2	23	8	92
	GP	25	0	25	0	100
3	SCM	25	0	25	0	100
	FCM	25	0	25	0	100
	GP	25	0.67	24.33	2.7	97.3
Average	SCM	25	0	25	0	100
	FCM	25	1	24	6.7	93.3

Cross-validation indicated that the models worked very well with different input data. Therefore, the overfitting could not be a problem and the models could be considered as comprehensive. In general terms, the outcomes presented in Table 6 and Fig. 7 display the remarkable possibility of the Subtractive Clustering Method with an average success rate of 100% in correctly foretelling the groutability of granular soil samples with the cement grout. However, other methods including Grid Partitioning and Fuzzy C-means clustering Method predicted the groutability of granular soil with success rate averages of 97.3% and 93.3%, respectively.



Fig. 7. Average failure and success rates of groutability prediction of the ANFIS models: (a) GP; (b) SCM and (c) FCM.

The performance of proposed models was investigated utilizing confusion matrix analyses. A confusion matrix is a performance measurement for machine learning classification problem where the output can be two or more classes. This approach uses a table with four different combinations of predicted and actual values (Fig. 8).







As seen in Fig. 8, several terminologies are used to calculate confusion matrix as follows: True Positive (TP) means that the model predicted positive values and it is true. True Negative (TN) indicates that the model predicted negative values and it is true. False Positive (FP) reveals that the model predicted positive values and it is false and finally, False Negative (FN) means that the model predicted negative is extremely useful for measuring Recall, Precision, Accuracy, and F-Means according to the following formulas:

$$\operatorname{Re} call = \frac{TP}{TP + FN}$$
(23)

$$Precision = \frac{TP}{TP + FP}$$
(24)

$$Accuracy = \frac{TP + TN}{Total}$$
(25)

$$F - Measure = \frac{2 \times \text{Recall} \times \text{Precision}}{\text{Recall} \times \text{Precision}}$$
(26)

The calculated parameters are presented in Table 7: Table 7. Confusion Matrix Atlases.

ANFIS Model	Recall	Precision	Accuracy	F-means
GP	0.63	1.00	0.96	0.77
SCM	0.60	1.00	1.00	0.75
FCM	0.64	0.93	0.88	0.76

As shown in Table 7, the results of confusion matrix analyses are in a high agreement with the previous results of the succession rate formula.

7. Sensitivity analysis

Sensitivity analyses were performed to determine the most effective input parameters on the output parameter. To achieve this goal, the cosine amplitude method (CAM) was used [46,47,59–62]. To apply this method, all data were declared in an X-space.

The data pairs were utilized to establish a data array X as described in Equation 27.

$$X = \{X_1, X_2, X_3, \dots X_m\}$$
(27)

Every member X_i in the data order X is a vector with the length of m, which is presented in Equation 28.

$$X_i = \{x_{i1}, x_{i2}, x_{i3}, \dots, x_{im}\}_1$$
(28)

Therefore, each data pair was considered as a point in an mdimensional domain, whereby every data requires m-coordinates for a comprehensive explanation. Every element of r_{ij} in Equation (25), presents a pairwise comparison of two data pairs.

The strength of the relation between the data pairs, x_i and x_j is described by the membership amount introducing the strength and is presented in Equation (29).

$$r_{ij} = \sum_{k=1}^{m} x_{ik} x_{jk} / \sqrt{\sum_{k=1}^{m} x_{ik}^2 \sum_{k=1}^{m} x_{jk}^2}$$
(29)

These strengths of relations' quantities, i.e. r_{ij} , between the input parameters and groutability for three types of the ANFIS models are illustrated in Fig. 9.

As illustrated in Fig. 8, the most effective variables on groutability were N1, N2, and P respectively. Furthermore, the least effective parameter on groutability was W/C.



Fig. 9. The r_{ij} between input parameters and groutability.

8. Conclusion

Three new ANFIS models were improved to predict the groutability in geoscience operations based on laboratory tests. The models were boosted on the strength of experts' knowledge and the datasets were collected from the literature. In this analogical investigation, three Takagi-Sugeno Fuzzy Inference Systems; GP, SCM, and FCM methods were applied to evaluate the groutability of granular soils . Some parameters included the proportion of water to cement in the grout (W/C), the relative density of soil (Dr), the soil percentage moving through a 0.6 mm sieve (FC), the injection pressure of grout (P), $Nl = D_{15 \text{ soil}} / d_{85 \text{ grout}}$ and $N2 = D_{10 \text{ soil}} / d_{95 \text{ grout}}$ were considered as inputs for models. To achieve favorable outputs, the training of ANFIS models using the datasets was carried out and the neuro-fuzzy parameters were obtained utilizing an ANN. The results of this study indicated that Subtractive Clustering Method with an average success rate and accuracy of 100% was the most effective method to foretell the groutability of granular soil samples in comparison with the other two methods. However, other methods including Grid Partitioning and Fuzzy C-means clustering with average success rates of 97.3% and 93.3%, respectively, were able to reasonably well foretell the groutability of granular soils. The comparison displayed the excellence of the ANFIS model on the strength of the SCM algorithm. In addition, it showed that the GP algorithm had better results in relation to the FCM algorithm. Furthermore, it was observed that the simulation results of the ANFIS models were close to the real measured values. Moreover, the three-fold cross validation indicated that the models worked very well with different input data. Therefore, overfitting cannot be considered as a problem and the models could be regarded as comprehensive. Consequently, sensitivity analyses were carried out using the cosine amplitude method (CAM) and indicated that based on the Grid Partitioning Method, the most effective factors in order of importance are N1, N2, and P respectively and W/C, FC and Dr respectively relate to the least effective parameter on groutability of granular soil. Moreover, according to the Subtractive Clustering Method, the most effective parameters in order of significance are N2, N1, and P respectively and W/C, Dr and FC are the least effective parameters on groutability, respectively. Finally, on the strength of the Fuzzy C-means clustering Method N1, N2, and P are respectively the most effective parameters and W/C, Dr and FC were the least effective parameters on the groutability granular soils . Although the three tested methods indicated the same parameters as the most/least effective parameters, the order of importance in each was different. The order of the most effective parameters in the GPM and FCM methods is N1, N2, and P respectively; however, the order of the least effective parameters in the SCM and FCM methods was W/C, Dr and FC.



REFERENCES

- Huang CL, Fan JC, Yang WJ. A study of applying microfine cement grout to sandy silt soil. Sino-Geotech 2007;111:71–82.
- [2] Dupla J-C, Canou J, Gouvenot D. An advanced experimental set-up for studying a monodirectional grout injection process. Proc Inst Civ Eng Improv 2004;8:91–9.
- [3] Gouvenot D. State of the art in European grouting. Proc Inst Civ Eng Improv 1998;2:51–67.
- [4] Jessberger HL. Soil Grouting general report: improvemente of ground. Proc. 8th Eur. Conf. soil Mech. Found. Eng., 1983, p. 1069–78.
- [5] Krizek RJ, Perez T. Chemical grouting in soils permeated by water. J Geotech Eng 1985;111:898–915.
- [6] Welsh JP. In situ testing for ground modification techniques. Use Situ Tests Geotech. Eng., ASCE; 1986, p. 322–35.
- [7] Akbulut S, Saglamer A. Estimating the groutability of granular soils: a new approach. Tunn Undergr Sp Technol 2002;17:371– 80.
- [8] Cambefort H. Foliate gravelly alluvium with opened structure. Proc. Fourth Conf. Large Dams (New Delhi, vol. 4, 1951, p. 434– 52.
- [9] Cambefort H. The principles and applications of grouting. Q J Eng Geol Hydrogeol 1977;10:57–95.
- [10] De Paoli B, Bosco B, Granata R, Bruce DA. Fundamental observations on cement based grouts (1): traditional materials. Proc Grouting, Soil Improv Geosynth New Orleans, La 1992:25– 8.
- [11] Incecik M, Ceran I. Cement grouting model tests. Bul Tek Univ 1995;48:305–18.
- [12] Johnson SJ. Cement and Clay Grouting of Foundations: Grouting with Clay-Cement Grouts. J Soil Mech Found Div 1958;84:1–12.
- [13] Karol RH, Swift AM. Symposium on Grouting: Grouting in Flowing Water and Stratified Deposits. J Soil Mech Found Div 1961;87:125–48.
- [14] Kennedy TB. Research in Foundation Grouting with Cement. Symp. Grouting, Transactions of the American Society of Civil Engineers; 1962, p. 1339–63.
- [15] King JC, Bush EGW. Symposium on Grouting: Grouting of granular materials. J Soil Mech Found Div 1961;87:1–32.
- [16] Kravetz GA. Cement and clay grouting of foundations: The use of clay in pressure grouting. J Soil Mech Found Div 1958;84:1– 30.
- [17] Markou IN, Atmatzidis DK. Development of a pulverized fly ash suspension grout. Geotech Geol Eng 2002;20:123–47.
- [18] Bezdek JC. Fuzzy mathematics in pattern classification. Ph D Thesis, Appl Math Center, Cornell Univ 1973.
- [19] Naudts A, Landry E, Hooey S, Naudts W. Additives and admixtures in cement-based grouts. Grouting Gr. Treat., 2003, p. 1180–91.
- [20] Raffle JF, Greenwood DA. The relation between the rheological characteristics of grouts and their capacity to permeate soil. Proc. 5th Int. Conf. Soil Mech. Found. Eng. Paris, vol. 2, 1961, p. 789.
- [21] Mitchell JK. In-place treatment of foundation soils. J Soil Mech

Found Div 1970;96:73-110.

- [22] Herndon J, Lenahan T. GROUTING IN SOILS VOLUME 2: DESIGN AND OPERATIONS MANUAL 1976.
- [23] Mitchell JK. Soil improvement: state-of-the-art: Department of Civil Engineering. University of California; 1981.
- [24] Sherard JL, Dunnigan LP, Talbot JR. Basic properties of sand and gravel filters. J Geotech Eng 1984;110:684–700.
- [25] De Paoli B, Bosco B, Granata R, Bruce DA. Fundamental observations on cement based grouts (2): Microfine cements and the Cemill[®] process. Grouting, Soil Improv. Geosynth., ASCE; 1992, p. 486–99.
- [26] Koronakis N, Kontothanassis P, Katsaris D, Bournazos J. Design of water isolation grouting for reducing high water inflows in urban shallow tunnels. Undergr. Sp. Use. Anal. Past Lessons Futur. Two Vol. Set Proc. Int. World Tunn. Congr. 31st ITA Gen. Assem. Istanbul, Turkey, 7-12 May 2005, CRC Press; 2005, p. 271.
- [27] Akbulut S, Saglamer A. Modification of hydraulic conductivity in granular soils using waste materials. Waste Manag 2004;24:491–9.
- [28] Kim J, Lee I, Jang J, Choi H. Groutability of cement-based grout with consideration of viscosity and filtration phenomenon. Int J Numer Anal Methods Geomech 2009;33:1771–97.
- [29] Chern S-G, Lee C-Y, Wang C-C. CPT-based liquefaction assessment by using fuzzy-neural network. J Mar Sci Technol 2008;16:139–48.
- [30] Chern S-G, Lee C-Y. CPT-based simplified liquefaction assessment by using fuzzy-neural network. J Mar Sci Technol 2009;17:326–31.
- [31] Dagdelenler G, Sezer EA, Gokceoglu C. Some non-linear models to predict the weathering degrees of a granitic rock from physical and mechanical parameters. Expert Syst Appl 2011;38:7476–85.
- [32] Garcia SR, Romo MP, Botero E. A neurofuzzy system to analyze liquefaction-induced lateral spread. Soil Dyn Earthq Eng 2008;28:169–80.
- [33] Gokceoglu C, Yesilnacar E, Sonmez H, Kayabasi A. A neurofuzzy model for modulus of deformation of jointed rock masses. Comput Geotech 2004;31:375–83.
- [34] Iphar M, Yavuz M, Ak H. Prediction of ground vibrations resulting from the blasting operations in an open-pit mine by adaptive neuro-fuzzy inference system. Environ Geol 2008;56:97–107.
- [35] Jalalifar H, Mojedifar S, Sahebi AA, Nezamabadi-Pour H. Application of the adaptive neuro-fuzzy inference system for prediction of a rock engineering classification system. Comput Geotech 2011;38:783–90.
- [36] Kucuk K, Aksoy CO, Basarir H, Onargan T, Genis M, Ozacar V. Prediction of the performance of impact hammer by adaptive neuro-fuzzy inference system modelling. Tunn Undergr Sp Technol 2011;26:38–45.
- [37] Rahman MS, Wang J. Fuzzy neural network models for liquefaction prediction. Soil Dyn Earthq Eng 2002;22:685–94.
- [38] Singh TN, Sinha S, Singh VK. Prediction of thermal conductivity of rock through physico-mechanical properties. Build Environ 2007;42:146–55.
- [39] Yilmaz I, Kaynar O. Multiple regression, ANN (RBF, MLP) and ANFIS models for prediction of swell potential of clayey soils.

Expert Syst Appl 2011;38:5958-66.

- [40] Yilmaz I, Yuksek G. Prediction of the strength and elasticity modulus of gypsum using multiple regression, ANN, and ANFIS models. Int J Rock Mech Min Sci 2009;46:803–10.
- [41] Liao K-W, Fan J-C, Huang C-L. An artificial neural network for groutability prediction of permeation grouting with microfine cement grouts. Comput Geotech 2011;38:978–86.
- [42] Tekin E, Akbas SO. Artificial neural networks approach for estimating the groutability of granular soils with cement-based grouts. Bull Eng Geol Environ 2011;70:153–61.
- [43] Pantazopoulos IA, Atmatzidis DK. Dynamic properties of microfine cement grouted sands. Soil Dyn Earthq Eng 2012;42:17–31.
- [44] Zadeh LA. Fuzzy sets. Fuzzy Sets, Fuzzy Logic, Fuzzy Syst. Sel. Pap. by Lotfi A Zadeh, World Scientific; 1996, p. 394–432.
- [45] Zimmermann HJ. Fuzzy set theory—and its applications: Springer Science & Business Media. New York 2001.
- [46] Ross TJ. Fuzzy logic with engineering applications. John Wiley & Sons; 2009.
- [47] Jang J-SR, Sun C-T, Mizutani E. Neuro-fuzzy and soft computing; a computational approach to learning and machine intelligence 1997.
- [48] Kasabov NK. Foundations of neural networks, fuzzy systems, and knowledge engineering. Marcel Alencar; 1996.
- [49] El-Shayeb Y, Verdel T, Didier C. Fuzzy Reasoning for the analysis of risks in geotechnical engineering Application to a French Case. 5. Eur. Congr. Intell. Tech. Soft Comput., 1997.
- [50] Takagi T, Sugeno M. Fuzzy identification of systems and its applications to modeling and control. Readings Fuzzy Sets Intell. Syst., Elsevier; 1993, p. 387–403.
- [51] Sugeno M, Kang GT. Structure identification of fuzzy model.

Fuzzy Sets Syst 1988;28:15-33.

- [52] Jang J-S. ANFIS: adaptive-network-based fuzzy inference system. IEEE Trans Syst Man Cybern 1993;23:665–85.
- [53] Lin C-T, Lee CSG. Neural-network-based fuzzy logic control and decision system. IEEE Trans Comput 1991;40:1320–36.
- [54] Chiu SL. Fuzzy model identification based on cluster estimation. J Intell Fuzzy Syst 1994;2:267–78.
- [55] Kasabov N, Zhou QQ, Purvis M. A membership function selection method for fuzzy neural networks 1997.
- [56] Tekin E, Mollamahmutoglu M. The Groutability of Microfine Cement (Rheocem 900) Grouts Into Various Graded Sands. J Fac Eng Archit GAZI Univ 2010;25:533–9.
- [57] Tekin E. Experimental studies on the groutability of microfine cement (Rheocem 900) grouts to sands having various gradations. Gazi Univ Google Sch 2004.
- [58] Zebovitz S, Krizek RJ, Atmatzidis DK. Injection of fine sands with very fine cement grout. J Geotech Eng 1989;115:1717–33.
- [59] Grima MA. Neuro-fuzzy modeling in engineering geology. AA Balkema, Rotterdam 2000;244.
- [60] Asadizadeh M, Hossaini MF. Predicting rock mass deformation modulus by artificial intelligence approach based on dilatometer tests. Arab J Geosci 2016;9. doi:10.1007/s12517-015-2189-5.
- [61] Asadizadeh M, Hossaini MF, Moosavi M, Mohammadi S. A laboratory study on mix design to properly resemble a jointed brittle rock. Int J Min Geo-Engineering 2016;50:201–10. doi:10.22059/ijmge.2016.59830.
- [62] Mohammad R, Mostafa A, Abbas M, Mohammad Farouq H. Prediction of representative deformation modulus of longwall panel roof rock strata using Mamdani fuzzy system. Int J Min Sci Technol 2015;25:23–30. doi:10.1016/j.ijmst.2014.11.007.